**Roll No:** 



# MARRI LAXMAN REDDY INSTITUTE OF TECHNOLOGY AND MANAGEMENT

(Approved by AICTE, New Delhi & Affiliated to JNTUH, Hyderabad)

Accredited by NBA and NAAC with 'A' Grade & Recognized Under Section2(f) & 12(B)of the UGC act, 1956

### III B.Tech I Sem Regular End Examination, January 2022

## Geotechnical Engineering (CIVIL)

Time: 3 Hours.	Max. Marks: 70

Note: 1. Question paper consists: Part-A and Part-B.

- 2. In Part A, answer all questions which carries 20 marks.
- 3. In Part B, answer any one question from each unit. Each question carries 10 marks and may have a, b as sub questions.

#### PART- A

 $(10 \times 2 \text{ Marks} = 20 \text{ Marks})$ 

1.	a)	Write about dispersive structure of soil	2M	CO1	BL2
	b)	Define dry unit weight and unit weight of dry soil solids	2M	CO1	BL1
	c)	Define Darcy's law.	2M	CO2	BL1
	d)	d) Define seepage velocity and write relation between porosity and seepage velocity.			BL2
	e)	Write benefits of soil compaction.	2M	CO3	BL2
	f)	Define pressure bulb and write its uses.	2M	CO3	BL1
	g)	What is normally consolidated soil?	2M	CO4	BL2
	h)	Define double drainage system and its maximum drainage path.	2M	CO4	BL1
	i)	How soil derives its shear strength?	2M	CO5	BL2
	j)	Define critical void ratio.	2M	CO5	BL1

#### PART-B

 $(10 \times 5 \text{ Marks} = 50 \text{ Marks})$ 

BL4

2	a)	Derive the fundamental equation in soil mechanics, $es = Gw$ by	5M	CO1	BL2
		defining e, s, G and w.	5M	C01	BL3

### OR

A soil sample with a grain specific gravity of 2.67 was filled in a 1000 ml container in the loosest possible state and the dry weight of the sample was found to be 14.75 N. It was then filled at the densest state obtainable and the weight was found to be 17.70 N. The void ratio of the soil in the natural state was 0.63. Determine the density index in the natural state. Also comment whether the insitu soil needs improvement or not.

4	a)	How void rat	io and particle s	size affects the p	permeability of soil.	5M	CO2	BL2
	b)	Calculate the	0 cm <sup>2</sup> in cross-so ml passed down	rmeability of a s ectional area, if a n in 10 minutes,	oil sample, 6 cm in a quantity of water under an effective	5M	CO2	BL3
				OR		10M	200	DI 4
5	•	Determine the average horizontal and vertical permeability of a soil mass made up of three horizontal strata, each 1 m thick, if the coefficients of permeability are $1 \times 10-1$ mm/s, $3 \times 10-1$ mm/s, and $8 \times 10-2$ mm/s for the three layers.					CO2	BL4
6	a)		Zero Air Void Li	ine is drawn alo	ng with compaction	5M	CO3	BL2
	b)	writing the tests, compaction c	are their com	standard and n paction energie	nodified compaction es and associated	5M	CO3	BL4
				OR				
7		How you cons	struct New marks	s chart and discus	s its application.	10M	CO3	BL4
8	a)	Describe the	limitations of 1-D	Consolidation th	eory.	5M	CO4	BL2
	b)	which the following regults have been obtained				5M	CO4	BL3
				OR				
9		In a laboratory, the consolidation test was performed on a specimen of clay 3cm thick. The sample was drained at top and bottom. The time required for 50% consolidation of the sample was observed to be 15 minutes. Determine the coefficient of consolidation of clay.				10M	CO4	BL3
1	0 a)	Discuss when we consider UU, CU and CD tests in geotechnical			5M	CO5	BL2	
		analysis.  b) Calculate the potential shear strength on a horizontal plane at a depth of 3 m below the surface in a formation of cohesionless soil when the water table is at a depth of 3.5 m. The degree of saturation may be taken as 0.5 on the average. Void ratio = 0.50; grain specific gravity = 2.70; angle of internal friction = 30°. What will be the modified value of shear strength if the water table reaches the ground surface?  OR					CO5	BL4
0.00		ml . c 11	- data valata ta a		ssion tests nerformed	10M	CO5	BL4
1	1	The following data relate to a triaxial compression tests performed on a soil sample:			2011			
		Test Np.	Chamber pressure	Max. deviator stress	Pore pressure at maximum deviator stress			
		1	80 kN/m <sup>2</sup>	175 kN/m <sup>2</sup>	45 kN/m <sup>2</sup>			
		, ,	150 kN/m²	240 kN/m <sup>2</sup>	50 kN/m <sup>2</sup>			

240 kN/m<sup>2</sup>

300 kN/m<sup>2</sup>

50 kN/m<sup>3</sup>

 $60 \text{ kN/m}^2$ 

2

3

150 kN/m<sup>2</sup>

 $210 \text{ kN/m}^2$